

1 **SCREW FASTENER IN MULTI-PLY WOOD STRUCTURE SHEAR
CONNECTION**

Background of the invention

 This application is a continuation-in-part of pending application serial
5 number 09/487,981, filed January 20, 2000 which was based on U.S. Ser.
No. 08/920,417 now U.S. 6,109,850 granted Aug. 29, 2000.

 In regions of the country subject to earthquake and hurricane events, it
is becoming increasingly common to provide metal straps and anchors for
affixing wood frame structures to their concrete foundations. For many
20 years manufacturers such as Simpson Strong-Tie Company, Inc. have
designed holdowns for use with bolts. (see Simpson catalog, January 1,
1996 pages 14 and 15 which illustrates holdowns such as HD2A covered by
U.S. Patent 4,665,672). These holdowns are very effective, but their load
rating is affected by the material which must be drilled out of the wood studs
25 in order to receive the stud bolts which range in diameter from 5/8" up to
1". Moreover, some undesirable looseness is inherent in the connection by
contractors who may inadvertently overdrill the bolt holes, or simply by the
fact that wood shrinkage occurs after installation of the bolts.

 The use of nails instead of bolts in holdowns has greatly reduced the
20 shrinkage and looseness problem of bolts and has led to the development of
strap holdowns as illustrated on pages 20, 22, and 23 e.g. of the Simpson
catalog supra, (see e.g. U.S. 5,150,553.) The problem with holdowns
which use nails is the fact that they must be very long to accommodate the
many nails that are required. See e.g. Simpson catalog page 23 in which the
25 HPAHD22-2p requires 23 -16d nails and may be 22" to 42" in length.
Many contractors now use nailing guns to drive the nails, but for the person
who does not have a nailing gun, the prospect of driving 23 nails for each
strap holdown means the expenditure of a great deal of energy driving the
nails.

30 With the increasing use of powered drills, the feasibility of using wood
screws as fasteners instead of nails and bolts is now a reality. The problem
with screws, particularly for large loading in shear is that standard screws
have several weaknesses. First, it was found that the heavy duty power
drivers snapped the heads off a high percentage of standard screws before
35 the clutch disengaged the drive at the end of the driving cycle when the head
abruptly reached the immovable sheet metal connector plate. Second, those

- 1 screws which had adequate unthreaded shank portions to resist the large
shear loads, split the wood upon installation or shortly thereafter because
the diameters of the unthreaded portions were larger than the bore made by
the threaded portion of the screw. Third, adequate self drilling features
5 were difficult to find in large size wood screws. Finally, existing screw
fasteners with unthreaded portions adjacent the head which had smaller
diameters to prevent wood splitting, were too loose. Looseness in standard
screw fasteners between the unthreaded shank and the side of the bore hole
which are subject only to pull out, is not a problem. Looseness, between the
10 unthreaded shank portion and the side of the bore hole is a major problem
when the screw fastener is subject to shear loads; particularly when the
shear loads are cycling loads as they are in earthquakes and hurricanes. In
such situations, each reversal of the shear loading tends to widen the bore
opening until major loosening occurs and now the loads are impact loads
15 which endanger the structure due to wood splitting.

In multi-ply wood structural connections where wood trusses, wood
joists, wood beams, engineered wood members or other wood structural
members were joined in load sharing connections, one practice was to join
the wood members with bolts inserted into bore holes formed in the wood.

- 20 This practice weakened the wood members because of the wood material
removed in forming the bolt holes. More importantly, a tight fit between the
bolts and wood was difficult to achieve initially, and nearly impossible to
maintain due to wood shrinkage. Looseness in such bolted connections
resulted in a loss of load sharing ability, leading to structural failures,
25 particularly in cyclic load reversals present during seismic or wind generated
occurrences.

Truss brackets such as U.S. 5,653,079 have been used to join trusses
together but these brackets are expensive, difficult to install and thus far
limited to connections between the narrow edges of wood chords rather than
30 the broad faces of the chords.

In three ply wood structural members, such members were normally
joined by nailing.

1 Summary of the invention

This application describes a wood screw which solves the aforesaid problems. First, a higher strength steel was used in the wood screw of the present invention.

5 Second, the wood screw of the present invention is formed with a cutting means at the entering end so that bore holes need not be predrilled.

Finally, the major problem of looseness between the sides of the fastener and the bore hole has been solved by the use of a knurled section which functions in a unique manner described herein below.

10 The use of the wood screw of the present invention solves the problems introduced by bolts by eliminating the need to predrill large openings in the wood which weaken the wood member in tension as introduced by earthquake and hurricane loadings.

The use of the wood screw of the present invention solves the
15 problem introduced by nails by enabling the strap connection to the wood frame to be significantly reduced in length thus saving in metal costs and installation problems.

The wood screw of the present invention is primarily for connecting wood structural members to sheet metal connectors in shear, but may also
20 be used with heavy metal members or even wood to wood connections.

This application is specifically directed to the use of the special screws of the present invention in joining the top and bottom wood chords and other truss members in multi-ply wood trusses. Such a connection obviates the problems formerly experienced in joining multi-ply trusses by boring bolt
25 openings and inserting bolts. Such a procedure also obviates the problems of using sheet metal connectors which are expensive to make and even more expensive to install. In some applications, the screws replace the use of nails. Primarily, multi-ply trusses joined by screws of the present invention, far out perform multi-ply trusses joined by sheet metal connectors in sharing
30 loads through events such as earthquakes and hurricanes.

Still another use of the screws of the present invention is to join multi-ply wood members in load sharing shear structural connections.

1 Brief Description of the Drawings

FIG. 1 is a side elevational view of a wood screw which is representative of one embodiment of the present invention:

~~FIG. 2 is an enlarged scale, partial central sectional view of the~~

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5 fastener shown in FIG. 1 in operative association with a portion of a wood structural member and a portion of a sheet metal member. Portions of the wood screw have been cut to indicate portions of the axial length of the wood screw have been removed so that the wood screw may meet the drawing paper restrictions. The upper portion of the wood screw is only
10 partially in cross section to clarify the details of the invention. In this view, the pointed end portion 7 and substantially all of the threaded shank portion 8 has moved through the opening in the sheet metal member 5 and entered the wood structural member 2. The knurled section 14 has not yet entered
the bore opening 3.

FIG. 1
FIG. 2
FIG. 3
FIG. 4
FIG. 5
FIG. 6
FIG. 7
FIG. 8
FIG. 9

15 FIG. 3 is partial central sectional view of the wood screw shown in FIGS. 1 and 2 in which the knurled portion 14 has just passed through the opening in the sheet metal member 5 and has entered the wood structural member 2. A portion of the knurled means 14 has been removed to show how the portions between the knurls fill up with mashed wood fibers from
20 the wood structural member.

FIG. 4 is a partial central sectional view of the wood screw shown in FIGS 1, 2 and 3 in which the wood screw is fully installed.

FIG. 5 is a side cross sectional view of the screw shown in FIG. 1 installed in a typical installation. A foundation to frame sheet metal
25 connector is illustrated connecting a wood frame member to a concrete foundation..

FIG. 6 is a cross sectional view of another use of the wood screws of the present invention.

Fig. 7 is an example of a typical truss profile.

30 Fig. 8 is an enlarged portion of a truss chord of the truss illustrated in Fig. 7 illustrating a typical spacing of the screws.

Fig. 9 is a cross section of the truss chord illustrated in Fig. 7. As an example, the screws are driven into one face of the truss chord. In some applications the screws may be driven from both sides.

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1 Fig. 10 is an example of a cross section of a multi-ply beam illustrating an existing sawn beam sistered by two engine red wood members such as Microlam® members.

Fig. 11 is an example of an assembly consisting of two 1 3/4" members joined by 1/4" x 3 1/2" screws. Other examples are illustrated in Figs. 12 - 16.

Fig. 12 is an assembly consisting of three 1 3/4" members joined by 1/4" x 3 1/2" screws.

Fig. 13 is an assembly consisting of four 1 3/4" members joined by 1/4" x 6" screws.

Fig. 14 is an assembly consisting of one 1 3/4" member and one 3 1/2" member joined by 1/4" x 3 1/2" screws.

Fig. 15 is an assembly consisting of two 1 3/4" members and one 3 1/2" member joined by 1/4" x 3 1/2" screws.

15 Fig. 16 is an assembly consisting of two 3 1/2" members joined by 1/4" x 6" screws.

Description of the Invention

Referring to the drawings, and in particular FIG. 1, the wood screw 1 of the present invention is adapted to hold a wood structural member 2 formed with a first bore 3 to a sheet metal member 5 in shear.

While the wood screw 1 of the present invention has excellent pull out value, the design is primarily directed to resisting shear forces. Two examples of environments in which the wood screw of the present invention is subject to shear forces are illustrated in the drawings.

25 Referring to FIG. 5, wood screw 1 connects a retrofit holdown device 36 to a wood sill member 37 resting on concrete foundation 38. Wood screw 1 is inserted through opening 45' in sheet metal member 5'. A bolt 39 connects the retrofit holdown device 36 to foundation 38. Arrow 40 represents an upward force exerted on wood sill member 37 which may occur during either an earthquake or a high wind force such as a hurricane. Such an upward force as represented by arrow 40 exerts a shear force along shear plane 41 as shown in FIG. 5. As may be understood, a force acting in the direction of arrow 42 exerts a pull out force on wood screw 1.

35 Another example of shear forces exerted on wood screws 1 of the present invention is illustrated in FIG. 6. Here, a sheet metal holdown 43 is connected to a foundation 38' by anchor bolt 39' and securely holds wood

- 1 sill member 37' to foundation 38'. Wood screws 1 of the present invention are inserted through openings 45" in sheet metal member 5" of holdown 43 into wood stud member 46. Arrow 40' r presents an upward force imposed by an earthquake or high winds such as a hurricane which imposes a shear
- 5 load along shear plane 41'. Arrow 42' represents a horizontal load imposed by an earthquake or high winds such as a hurricane which imposes a pullout force on wood screws 1.

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- Referring now in detail to the wood screw 1 of the present invention, as most clearly shown in FIGS. 1 and 2, the screw 1 includes; a shank 6
- 10 having an overall length 44; a pointed end portion 7 formed on an entering extremity of the shank 6; the shank 6 having a threaded shank portion 8 having thread convolutions 9 with an outer diameter 10 greater than the diameter of the first bore 3 and beginning at a first point 11 adjacent the pointed end portion 7 and extending axially along the periphery of the shank
- 15 6 to a second point 12 and adapted to form and engage threads 13 in the wood structural member 2; knurled means 14 formed in a portion of the shank 6 having a first point 15 adjacent the second point 12 of the threaded shank portion 8 and extending axially along the shank 6 to a second point 16 and having an outside diameter 17 generally equal to the outer diameter
- 20 10 of the thread convolutions 9 in the threaded shank portion 8 and having an inside diameter 18 (see FIG. 4) less than the outside diameter 17 of the knurled means 14; the shank 6 having an unthreaded shank portion 19 having a diameter 20 generally equal to the outside diameter 17 of the knurled means 14 and having a first point 21 adjacent the second point 16
- 25 of the knurled means 14 and extending axially along the shank 6 and terminating at a second point 22; the knurled means 14 being adapted for mashing over and radially outwardly without severing a substantial proportion of the wood fibers of the inner portions 23 of the threads 13 formed in the wood structural member 2 forming an annular zone 55 of
- 30 mashed and severed, as well as unsevered wood fibers having an outer diameter 56 greater than the diameter 20 of the unthreaded shank portion 19 and forming a tight fit between the unthreaded shank portion 19 and the annular zone 55 of mashed and severed, as well as unsevered, wood fibers of the wood structural member 2; and a head 26 integrally connected to
- 35 the shank 6 adjacent the second point 22 of the unthreaded shank portion

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- 1 The wood screw of the present invention need not have a thread d
pointed end or a means for cutting its own bore and threads in a wood
m mber if a bore is predrilled. It is highly advantageous, however, to form a
wood screw 1 which will drill its own bore and threads in a wood member
5 since predrilling a bore is expensive in installation time. Power drivers to
drive large diameter wood screws are now widely available and thus,
referring to FIGS. 1 and 4, a preferred form of the wood screw 1 of the
present invention adapted to hold a wood structural member 2 to a sheet
metal member 5 in shear includes; a shank 6; a pointed end portion 7
10 formed on an entering extremity of the shank 6 having a plurality of
convolutions 27 and a recess 28 providing a cutting edge 29 adapted for
forming a first bore 3 having a diameter 4; and the shank 6 having a
threaded shank portion 8 having thread convolutions 9 similar to the thread
convolutions 27 on the pointed end portion 7 with an outer diameter 10
15 greater than the diameter 4 of the first bore 3 and beginning at a first point
11 adjacent the pointed end portion 7 and extending axially along the
periphery of the shank 6 to a second point 12 and adapted to form and
engage threads 13 in the wood structural member 2. All other elements of
the preferred form of wood screw 1 are identical to the previously described
20 wood screw and for purposes of brevity are not repeated.

~~Referring to FIG. 3, knurled means 14 may be double knurled in a
cross hatched pattern or have single straight knurls formed at an angle to the
axis of the screw. It has been found, however, that straight knurls 30 (see
FIG. 2) having a dull edge 47 and valleys 48 between the dull edges 47
25 perform satisfactorily.~~

~~It has also been found that where the axial length 31 (see FIG. 1) of
the knurled means 14 is substantially less than the axial length 32 of the
unthreaded shank portion 19 satisfactory results are obtained.~~

- 30 Providing wood screw 1 with an unthreaded portion 19 reduces the
power requirements to drive the screw and maximizes the amount of metal
at the shear plane 41 and 41' (see FIGS. 5 and 6) adjacent the head 26 of
the wood screw 1. Accordingly, the axial length 32 of the unthreaded
portion 19 is preferably substantially less than the axial length 33 of the
threaded portion 8.

- 35 To accommodate the power driven tool and provide maximum gripping
power, the head 26 is preferably hexagonal in shape.

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35 The function of the dull edges 47 of knurls 30 is to bend the inner portions 23 of threads 13 in the structural wood member so as to mash

1 ~~rather than to cover a substantial portion of the wood fibers of the structural~~
wood member. These bent over and mashed wood fibers as well as the
severed wood fibers are illustrated in FIGS. 3 and 4 and are indicated
generally by the number 55 which represents an annular zone of mashed and
5 severed, as well as unsevered, wood fibers. The annular zone of mashed
wood fibers 55, as seen when wood screw 1 is fully seated, is bounded by
the space outboard of diameter 20 of unthreaded shank portion 19 and
outer diameter 56 of the mashed fiber annular zone. Annular zone of
mashed wood fibers 55 as seen in FIG. 4 extends from outer face 53 of
10 wood structural member 2 to penetration point 61 (see FIG. 4) of upper
bevel end 59 of knurled means 14 (see FIG. 2).

As the knurled means 14 proceeds into the wood structural member 2,
the valleys 48 between the dull edge ridges 47 of knurls 30 fill with the
unsevered fiber ends as well as severed wood fibers of the mashed over
15 inner portions 23 of threads 13 and loose cuttings from the cutting edge 29
on the pointed end portion 7 of the wood screw 1. This filling of the valleys
48 in the knurls 30 further reduces the cutting or severing of the wood
fibers as the knurl means 14 continues through the wood structural
~~member 2.~~

20 The effects of the previously described mashing of the wood fibers is
shown in FIG. 4. In this view, the wood screw 1 has been fully inserted into
the structural wood member 2 and is now in place to resist shear forces
acting between sheet metal member 5 and the wood structural member 2.
The result of the wood mashing of inner portions 23 of threads 13 of the
25 wood structural member 2 is that the mashed wood fibers form an annular
zone 55 which tightly fills any space between the outer diameter 20 of
unthreaded shank portion 19 and the outer diameter 56 of the mashed fiber
annular zone 55. This annular zone 55 of tightly packed mashed wood fibers
mixed with some cuttings from cutting edge 29 on the pointed end portion 7
30 of the wood screw 1 prevents essentially all looseness between the wood
screw 1 and the structural member 2. This tight fit of the wood screw 1
with the structural wood member serves to increase the wood screws
resistance to lateral displacement which contributes to the increase in shear
resistance along the shear planes 41 and 41' as seen, e.g. in FIGS 5 and 6.

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35 ~~By sizing the knurled means 14 with an outside diameter 17 generally
equal to the outer diameter 10 of the thread convolutions 9 in the threaded~~

1 ~~shank portion 8 and generally equal to the diameter 20 of the unthreaded~~
~~shank portion 19, wood splitting as the unthreaded shank portion enters the~~
~~wood structural member 2 is obviated.~~

As an example, the wood screw 1 of the present invention may be
 5 manufactured from 1022 steel (SAE Grade 5) with a finish coat of zinc and
 dichromate. The hex washer head 26 may be 0.375 inch (9.5 mm). The
 self drilling tip or pointed end portion 7 may be a Type-17, and allows for
 driving without lead holes. Lead holes, however, may be required by the
 local building official, depending on wood type and moisture content in
 10 accordance with Section 2339.112 of the Code of International Conference
 of Building Officials (ICBO).

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~~Some typical dimensions of wood screws of the present invention~~
 having an overall shank length 44 measured from the underside 50 of washer
 35 to the point 51 ranging from 1 1/2" to 3 1/2" are as follows: The length
 15 33 of the threaded section 8 may vary from 7/8" to 3 1/4" while the axial
 length 31 of the knurled section 14 remains at a constant .250". and the
 length 32 of the unthreaded shank portion 19 varies with the length of the
 wood screw 1. For example, where the shank length is 1 1/2", the
 unthreaded shank portion 19 may be 5/8" whereas an overall shank length
 20 44 of 3 1/2" may have an unthreaded shank length 19 of 1 1/4". Outer
 diameter 10 of thread convolutions 9 may have a diameter of 0.259 - 0.250"
~~and an inner diameter of 0.187" to 0.183"~~

While the wood screw of the present invention is shown in FIGS. 5
 and 6 for use with holdown connectors used in attaching wood frame
 25 buildings to concrete foundations, the wood screw as above described may
 be used anywhere that wood screws of the size and type described may be
 used. The wood screws of the present invention may be used with heavy
 metal members or wood to wood connections.

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Referring to FIGS. 7, 8, and 9, a truss 70 is illustrated having top
 30 chords 71 and 72, web members 76 and a bottom chord 73. To share
 loads, additional trusses may be placed side by side and at least one of the
 chords of each adjacent truss should be wood and joined by screws 1. As
 illustrated, all of the chords 71 - 73 are wood and all of the multi-ply chords
~~71-73 are joined by screws 1~~

35 Fig 8 illustrates the typical spacing of the screws with screws 1'
 forming a row near the upper edge of a wood chord such as the bottom

1 chord 73 of truss 70. In the bottom chord, the screws in the upper row are indicated by the number 1' and the screws in the lower row are indicated by the number 1". Each of the multi-ply chords 73', 73", 73'", and 73''' making up the bottom chord 73, as shown in Fig. 9, are joined by screws 1' and 1" in shear.

The screws 1 are staggered and edge distance 75, as illustrated in Fig. 8, must meet edge distances required by the codes. The spacing 74 varies according to the loads.

In a design example of a 3 or 4 ply girder truss, the bottom chord 73 may be 2 x 6 Douglas Fir-Larch and the top chords 71 and 72 2 x 4 Douglas Fir Larch. For such a truss the total load on the bottom chord for example might be 500 pounds per lineal foot. The allowable load on each screw 1 could be 340 pounds for a roof live load condition. Spacing of the screws 1 might be 16" on center maximum for this example.

For the top chord in this example, one row of screws was used with spacing of 24" on center.

The screws in this example are $\frac{1}{4}$ " x 4 1/2" for a 3 ply truss and $\frac{1}{4}$ " x 6" for a 4 ply truss.

In installing the screws of the present invention it may be noted that: No pre-drilling is required, but predrilling may be permitted in retrofit applications, for instance, where the wood is very dry.

Screws may be installed from one side of the truss, as illustrated in Fig. 9, for faster fabrication thereby eliminating the need to flip the truss. As shown in Fig. 13, in some applications, the screws may be driven from both sides.

Screws must be installed in the same truss ply that the hangers are attached to for best results.

Screws may be used to field-join trusses if specified by the Engineer.

Screw location and minimum spacing must follow the requirements of the applicable design codes.

Screws must be installed in the bottom and top truss chords for best results, and may be installed in the truss webs if required by the Truss Engineer.

The screws should not be over driven.

~~All plies of the truss lateral bracing should normally be connected.~~

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1 Other general considerations in joining multi-ply wood trusses are as follows:

All screws must penetrate a minimum of 1 inch into the last truss ply for best results.

5 A maximum gap of 1/8 inch is allowed between each truss ply as long as the penetration required of one inch into the last ply is provided.

Spacing of screws shall not exceed 24 inches on center.

The Truss Engineer shall ensure that adequate lateral bracing is provided to prevent displacement of the truss and the truss bottom chord
10 due to the torsion created by the structural members framing into the side of the multi-ply girder truss.

If the screws are installed in the wrong face of the truss (the screws should be installed on the face of the truss where hangers are installed), then additional screws should be installed in the correct face with a
15 maximum spacing of 2 times the required spacing, not to exceed 24 inches on center. The additional screws shall be offset from the existing screws to prevent splitting. (This caution is especially true where screws are being installed in both faces of the truss).

The screws should not be installed through metal truss plates unless
20 approved by the Truss Engineer.

One row of screws should normally be used in 2 x 4 members; 2 rows in 2 x 6 members, and 3 rows in 2 x 10 members. Rows should be staggered.

Individual screw locations may be adjusted up to 1/2 of the required
25 screw spacing to avoid conflicts with other hardware or to avoid lumber defects.

Attaching multiple plies of structural composite lumber and sawn lumber with the screws of the present invention.

30 Referring to Figs. 11 - 16 of the drawings, installation of the screws of the present invention may be installed in multiple plies of composite engineered lumber such as Laminated Veneer Lumber (LVL), Parallam® (PSL) TimberStrand® sawn lumber or other engineered wood products.

Installation may be done by hand or power tools. If driven by power
35 tools, the screws of the present invention should not be over driven. The number and spacing of screws must be specified by an Engineer and is

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1 dependent upon loads, spacing of the loads, type of lumber serving as the structural lumber and placement of the loads.

Referring to Fig. 10, multiple plies of structural members are joined by screws of the present invention. The center member 77 illustrated may be a
5 sawn lumber ceiling joist found in an existing structure. If in a remodel of a building structure such as might occur if it was desired to remove posts and create a room with a clear span, or where it might be desirable for the ceiling joists to support greater loads, one or more sawn lumber members or engineered members such as one or more micro-lam® beams 78 and 79 or
10 other type of composite engineered wood beam may be "sistered" to the existing joist member 77. The special screws 1' and 1" of the present invention are most effective in this situation because of their unique ability to cause the structural wood members to share loads reliably under design loads for specified vertical loads, seismic events or wind load events.
15 Again, it is the special ability of the screws 1 to form tight fitting shear connections that makes this result possible.

Referring to Figs. 11 - 16, further examples of multiple plies of structural members are illustrated. All of the examples may be in either retrofit or new construction. For most reliable results as to design loads,
20 new materials of like material should be used.

In Fig. 11, two engineered lumber members, 80 and 81 are joined by an upper row of screws 1' of the present invention and a lower row of screws 1" of the present invention.

In Fig. 12, three engineered lumber members 82, 83, and 84 are
25 joined by an upper row of screws 1' of the present invention and a lower row of screws 1" of the present invention which are driven from one direction, and an upper row of screws 1" ' of the present invention and a lower row of screws 1" " of the present invention which are driven from the opposite direction.

30 In Fig. 13, four engineered lumber members 85, 86, 87, and 88 and are joined by an upper row of screws 1' of the present invention and a lower row of screws 1" of the present invention which are driven from one direction, and an upper row of screws 1" ' of the present invention and a lower row of screws 1" " of the present invention which are driven from the
35 opposite direction. Because of the greater thickness of the four plies, the

1 1/4" x 3 1/2" screws used in Figs. 11, and 12 are replaced by screws 1/4" x 6".

5 In Fig. 14, an example is shown in which structural member 90 is substantially of greater width than structural member 89. The two members are shown joined by screws of the present invention with the upper row of screws 1' being 1/4" x 3 1/2" and the lower rows of screws 1" being the same size and length.

10 In Fig. 15, an example is illustrated of a relatively wider structural member 91 "sistered" by relatively narrower structural members 92 and 93. In the example, screws of the present invention may be 1/4" x 3 1/2" arranged in an upper row of screws 1', a lower row of screws 1", and an upper row of screws 1" ' and a lower row of screws 1" " driven from the opposite side.

15 In the last example, Fig 16, two structural members 94 and 95, each 3 1/2" in thickness, are illustrated joined by screws of the present invention having a dimension of 1/4" x 6". An upper row of screws 1' and a lower row of screws 1" are driven from one direction and an upper row of screws 1'" and a lower row of screws 1" " are driven from the opposite direction

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